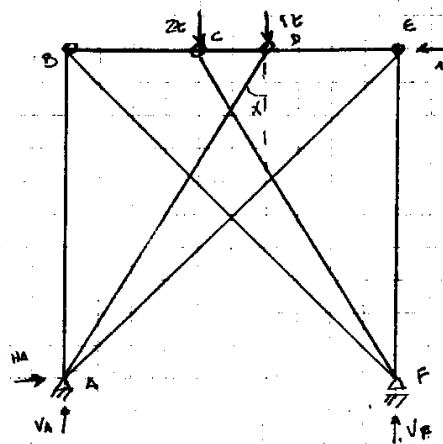


**Solución – Examen Febrero 2005 – Resistencia de Materiales 1 y 1N**

**Ejercicio 1**



Reacciones:

$$H_A = 1t$$

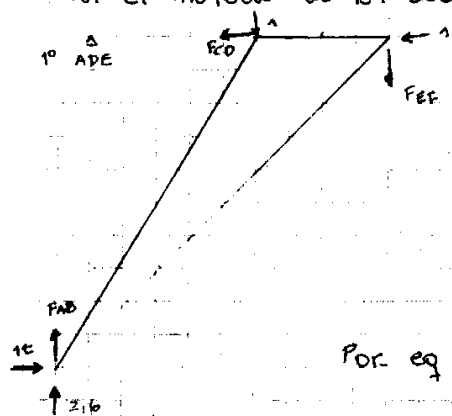
$$M_A = 0 : 5V_F + 1 \times 5 - 1 \times 3 - 2 \times 2 = 0$$

$$V_F = \frac{2}{5}$$

$$M_H = 0 : 5V_A - 2 \times 3 - 1 \times 2 - 1 \times 5 = 0$$

$$V_A = \frac{13}{5} = 2.6 \quad V_A + V_F = 3t \quad \checkmark$$

Por el método de las secciones corto: AB, CD y EF



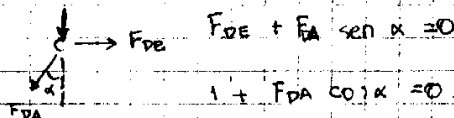
$$M_E = 0$$

$$(F_{AB} + 2.6) \times 5 - 1 \times 5 - 1 \times 2 = 0 \quad F_{AB} = -\frac{6}{5} = -1.2t$$

$$\text{Eq vertical: } F_{AB} + 2.6 - 1 - F_{EF} = 0 \rightarrow F_{EF} = 0.4t$$

$$\text{Eq horiz: } F_{CD} = 0$$

Por eq en el nodo D:



$$F_{DE} + F_{DF} \sin \alpha = 0$$

$$1 + F_{DA} \cos \alpha = 0$$

$$\tan \alpha = \frac{5}{3}$$

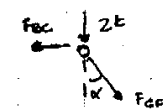
$$AD = 5.83$$

$$\sin \alpha = \frac{3}{5.83} = 0.51 \quad \cos \alpha = \frac{5}{5.83} = 0.86$$

$$\rightarrow F_{DA} = \frac{-1}{0.86} = -1.17$$

$$F_{DE} = -1.17 \times 0.51 = -0.6$$

Eq nodo C:

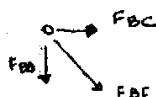


$$F_{bc} = F_{cc} \sin \alpha$$

$$F_{cc} \cos \alpha = -2 \rightarrow F_{cc} = -2.3$$

$$F_{bc} = -1.2$$

Eq nodo B:



$$F_{BF} = -(F_{bc} + F_{ba}) \frac{\sqrt{2}}{2} = 1.7t$$

$$\rightarrow \text{Eq E } F_{EA} = -((1 - 0.6)^2 + 0.4^2)^{1/2} = -0.57$$

$$F_{AB} = -1.2t$$

$$F_{AD} = -1.17t$$

$$F_{AE} = -0.57t$$

$$F_{BC} = -1.2t$$

$$F_{BF} = 1.7t$$

$$F_{CD} = 0$$

$$F_{CF} = -2.3t$$

$$F_{DE} = -0.6t$$

$$F_{EF} = 0.4t$$

(+) Tracción

(-) Compresión

DIMENSIONADO :

Máxima tracción = 1700 kg  $\sigma_{adm} = 1400 \text{ kg/cm}^2$

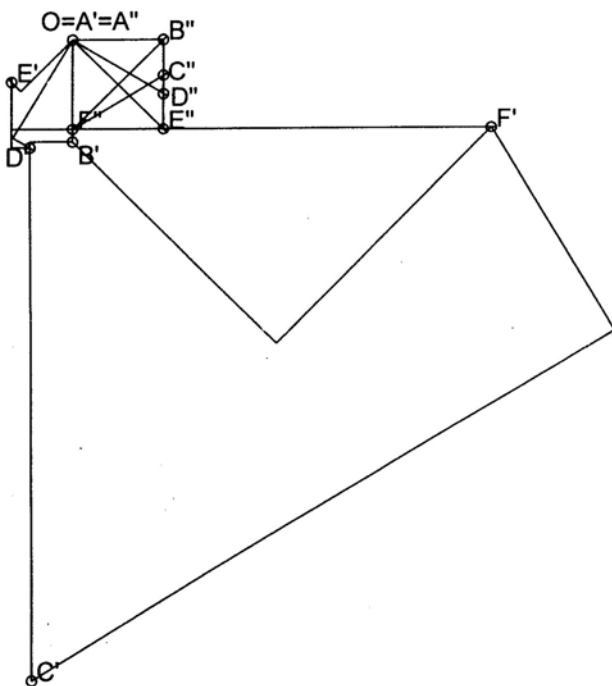
$$\sigma = \frac{F}{A} = \frac{F}{\pi d^2/4} \rightarrow d = \sqrt{\frac{4F}{\pi \sigma}} = 1,24 \text{ cm} \quad A = 1,21 \text{ cm}^2$$

Máxima compresión = 2300 kg  $\sigma_{adm} = 1400 \text{ kg/cm}^2$

$$\sigma = \frac{F}{d^2 - 0,8d^2} = \frac{F}{0,36d^2} \rightarrow d = \sqrt{\frac{F}{0,36\sigma}} = 2,13 \quad A = 1,64 \text{ cm}^2$$

DESPLAZAMIENTOS :  $\Delta L = \frac{PL}{EA}$

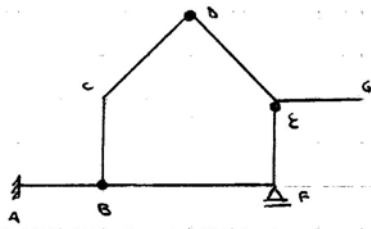
BARRA	F	L	A	$\Delta L$ (cm)
AB	-1200	500	1,64	-0,17
AD	-1170	583	1,64	-0,19
AE	-570	707	1,64	-0,12
BC	-1200	200	1,64	-0,069
BF	1700	707	1,21	0,47
CD	0	100	-	0
CF	-2300	583	1,64	-0,39
DE	-600	200	1,64	-0,03
EF	400	500	1,21	0,079



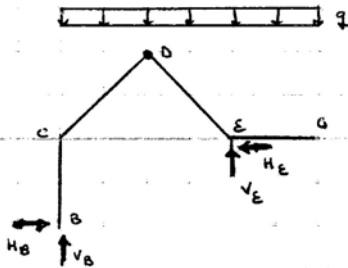
	$\delta_H$ (cm)	$\delta_V$ (cm)
A	0	0
B	-0,15	+0,17
C	-2,19	+1,00
D	-0,22	+0,09
E	-0,25	-0,08
F	+0,69	0

**Solución – Examen Febrero 2005 – Resistencia de Materiales 1 y 1N**

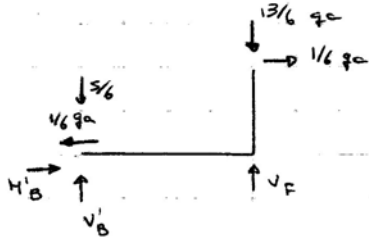
**Ejercicio 2**



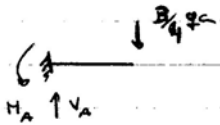
$$\left\{ \begin{aligned} H_B &= H_E \\ V_B + V_E &= 3qa \\ V_B 2a + H_B a &= \frac{3}{2} qa^2 \\ V_B a + H_B 2a &= \frac{9}{2} qa^2 \end{aligned} \right.$$



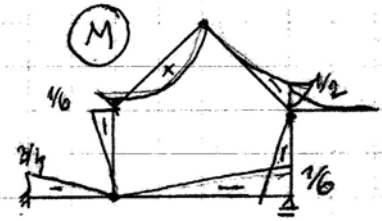
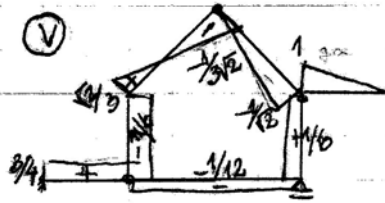
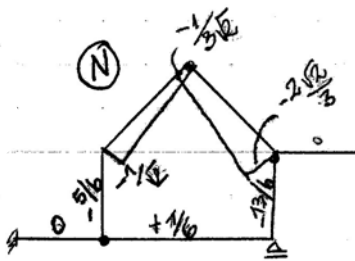
$$\begin{aligned} 2V_B + H_B &= 3/2 qa \\ -2V_B + 2H_B &= -qa \\ 3H_B &= 1/2 qa \\ H_B = H_E &= 1/6 qa \\ V_B = \frac{3qa}{2} + 2H_B &\Rightarrow V_B = \frac{5}{4} qa \\ V_E = 3qa - V_B &\Rightarrow V_E = \frac{13}{4} qa \end{aligned}$$



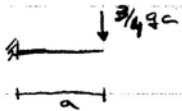
$$\left\{ \begin{aligned} H'_B &= 0 \\ V'_B + V_F &= 3qa \\ V_F 2a - \frac{3}{2} qa 2a - \frac{1}{2} qa a &= 0 \Rightarrow V_F = \frac{9}{4} qa \\ V'_B &= (3 - \frac{9}{4}) qa = \frac{3}{4} qa \end{aligned} \right.$$



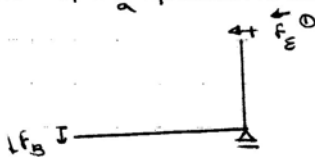
$$\left\{ \begin{aligned} V_A &= \frac{3}{4} qa \\ H_A &= \frac{3}{4} qa^2 \end{aligned} \right.$$



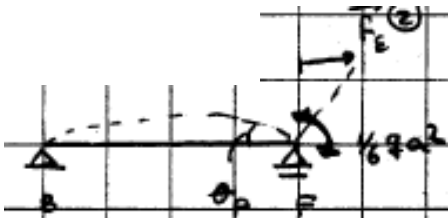
**Desplazamiento**



$$\downarrow f_B = \frac{3}{4} qa \frac{a^3}{3EI} = \frac{qa^4}{4EI}$$

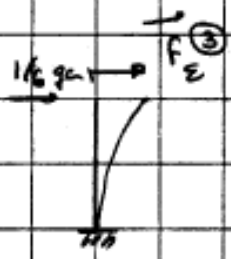


$$\leftarrow f_E = \frac{1}{2} f_B = \frac{qa^4}{8EI}$$



$$\theta_B = \frac{1}{6} qa^2 \frac{2a}{3EI} = \frac{qa^3}{9EI}$$

$$f_{B2} = \theta_B a = \frac{qa^3}{9EI} a = \frac{qa^4}{9EI}$$

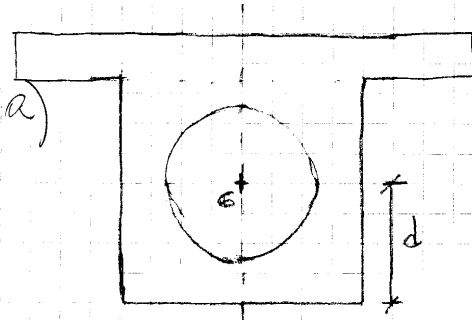


$$f_{B3} = \frac{1}{6} qa \frac{a^3}{3EI} = \frac{qa^4}{18EI}$$

$$f_B = -f_{B1} + f_{B2} + f_{B3} = \left( -\frac{1}{8} + \frac{1}{9} + \frac{1}{18} \right) \frac{qa^4}{EI} = \frac{qa^4}{24EI}$$

**Solución – Examen Febrero 2005 – Resistencia de Materiales 1 y 1N**

Ejercicio 3.



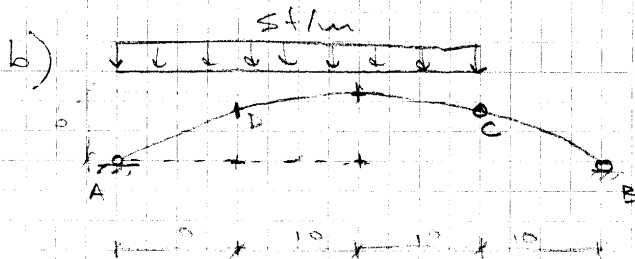
$$i) A_G = 150 \times 15 \times 82,5 + 77,5 \times 80 \times 75 - \pi \times 75^2 = d (15 \times 150 + 80 \times 75 - \pi \times 75^2)$$

$$d = 49,77 \text{ cm}$$

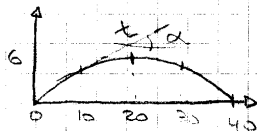
$$ii) I_x = 150 \times \frac{15^3}{12} + 150 \times 15 \times 32,5^2 + 80 \times \frac{75^3}{12} + 80 \times 75 \times 12,25^2 - \frac{\pi 75^4}{64}$$

$$I_x = 5.861.683 \text{ cm}^4$$

$$I_y = 15 \times \frac{150^3}{12} + 75 \times \frac{80^3}{12} - \frac{\pi 75^4}{64} = 7.112.104 \text{ cm}^4 = I_y$$



Parábola:



$$\begin{cases} y(x) = ax^2 + bx \\ y'(x) = 2ax + b \end{cases} \Rightarrow \begin{cases} y(20) = 400a + 20b = 6 \\ y'(20) = 40a + b = 0 \end{cases} \Rightarrow \begin{cases} a = -\frac{3}{200} \\ b = \frac{3}{5} \end{cases}$$

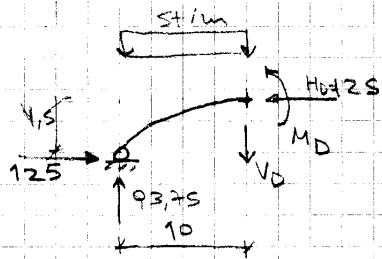
$$M_y(x) = -\frac{3}{200}x^2 + \frac{3}{5}x \Rightarrow M_C(10) = M_D(30) = 4,5 \text{ t}$$

$$M'_y(x) = -\frac{3}{100}x + \frac{3}{5} \Rightarrow M'_y(10) = 0,3 = \text{tg } \alpha \Rightarrow \alpha \approx 16,7^\circ$$

Reacciones:  $M_A = 5 \times 30 \times 5 - V_B \times 10 = 0 \Rightarrow V_B = 56,25 \text{ t}$

$$V_A = 5 \times 40 - 56,25 = 93,75 \text{ t} \cdot V_A$$

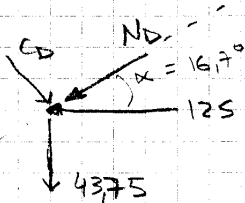
$$M_C^\perp = 4,5 \times H - 56,25 \times 10 = 0 \Rightarrow H = 125 \text{ t}$$



$$V_D = 93,75 - 5 \times 10 = 43,75 \text{ t}$$

$$M_D = -125 \times 4,5 - 5 \times 10 \times 5 + 93,75 \times 10$$

$$M_D = 125 \text{ t m}$$

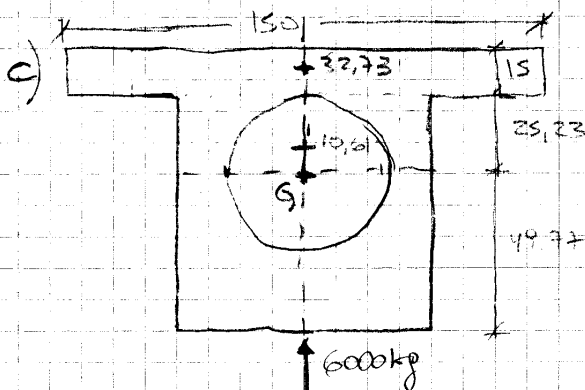


$$N_D = 125 \cos 16,7^\circ + 43,75 \sin 16,7^\circ$$

$$N_D = 132,3 \text{ t (comp.)}$$

$$C_D = -125 \sin 16,7^\circ + 43,75 \cos 16,7^\circ$$

$$C_D = 6,0 \text{ t}$$



$$M_G = 15 \times 150 \times 32,73 + 80 \times 25,23 \times 12,62 - \pi \frac{25^2}{2} \times 10,61$$

$$M_G = 88703,6 \text{ cm}^3$$

$$b = 80 - 50 = 30$$

$$\tau_G = \frac{V \cdot M}{b \cdot I} = \frac{6000 \text{ kg} \times 88703,6}{30 \times 5.861.683} = 3,0 \text{ kg/cm}^2$$