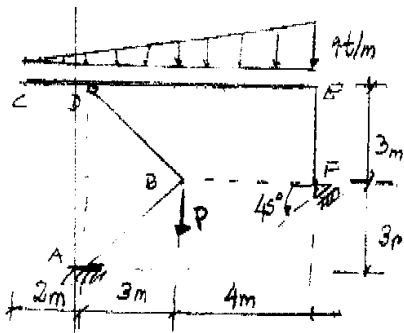
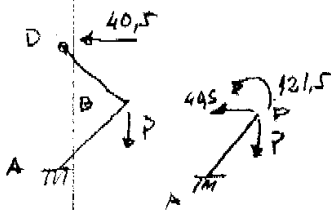
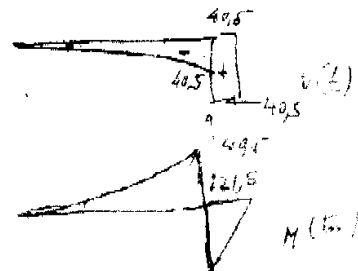
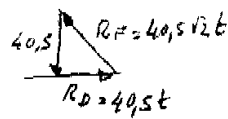
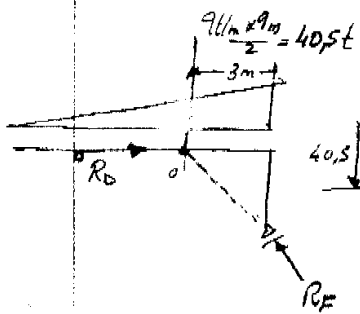


Ejercicio 1



Valor de P para el cual el giro en B es nulo.  
 Para ese P, reacciones, diagramas de solicitaciones y desplazamientos horizontal y vertical de D. (en función de EI)  
 Despreciar deformaciones por directa. EI = cte

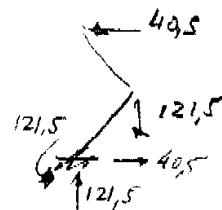
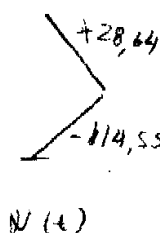
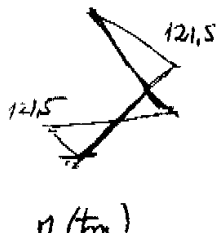
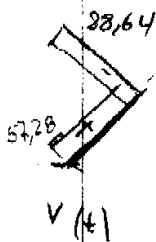


$$EI \theta_B = \frac{(P-40.5)(3\sqrt{2})^2}{2} - 121.5 \times 3\sqrt{2} = 0 \Rightarrow P = 121.5t$$

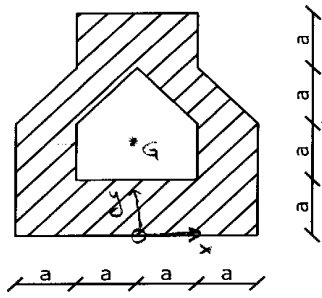
$$EI f_B \downarrow = \frac{(P-40.5)(\frac{3\sqrt{2}}{2})^3}{3} - 121.5 \times \frac{(3\sqrt{2})^2}{2} = 364.5 \quad EI f_B \downarrow = 0 \text{ (despreciamos def. x directa)}$$



$$EI f_D \downarrow = EI f_B \downarrow = 364.5 \quad ; \quad EI f_D \leftarrow = \frac{40.5}{\sqrt{2}} \frac{(3\sqrt{2})^3}{3} = 729 \quad \left. \begin{array}{l} f_D \downarrow = \frac{773.22}{EI} \\ f_D \leftarrow = \frac{2577.4}{EI} \end{array} \right\}$$



Ejercicio 2



$$M_x = 6a^2 \frac{5a}{2} + 4a^2 \frac{1}{2}a = 17a^3$$

$$A = 10a^2$$

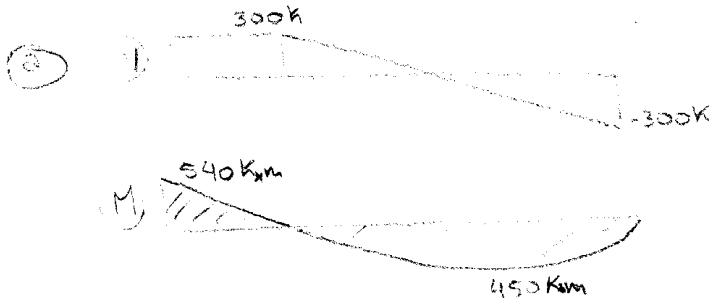
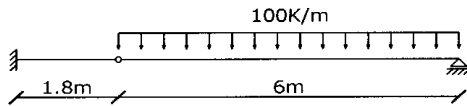
$$y_G = \frac{M_x}{A} = 1,7a$$

$$I_x = \frac{2a(3a)^3}{12} + 6a^2(0,8a)^2 + \frac{4a^3}{12} + 4a(1,2a)^2$$

$$I_x = 14,43a^4$$

$$W_{inf} = 8,49a^3$$

$$W_{sup} = 6,27a^3$$



b) • Para el mom. positivo  $\rightarrow \frac{450 \text{ Kcm}}{W_{inf}} \leq \sigma_{tracc}^{adh} = 100 \text{ K/cm}^2 \Rightarrow a \geq 3,76 \text{ cm}$

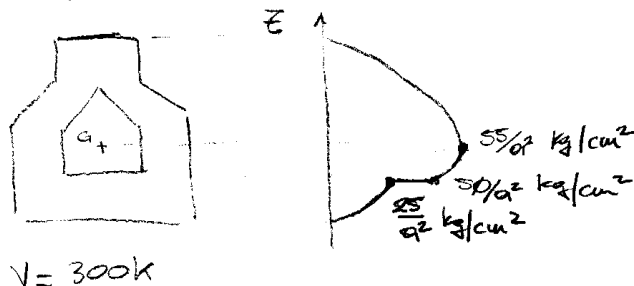
$\rightarrow \frac{450 \text{ Kcm}}{W_{sup}} \leq \sigma_{comp}^{adh} = 50 \text{ K/cm}^2 \Rightarrow a \geq 5,24 \text{ cm}$

• Para el mom. negativo  $\rightarrow \frac{540 \text{ Kcm}}{W_{sup}} \leq \sigma_{tracc}^{adh} \Rightarrow a \geq 4,41 \text{ cm}$

$\rightarrow \frac{540}{W_{inf}} \leq \sigma_{comp}^{adh} \Rightarrow a \geq 5,03 \text{ cm}$

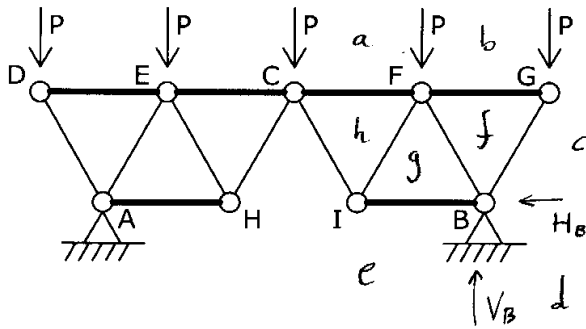
$$\boxed{a = 5,2 \text{ cm}}$$

c) La sección rasante máxima se da en toda la ménsula y la dist. de tensiones rasantes en dichas secciones es:



$$V = 300 \text{ K}$$

Ejercicio 3



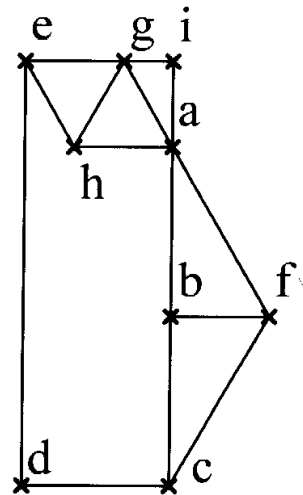
$$\sum M_A = 0 \Rightarrow 5P \cdot 1,5L = V_B \cdot 3L$$

$$\Rightarrow V_B = 2,5P$$

$$\sum M_C = 0 \Rightarrow (2,5 - 2)P \cdot 1,5L = H_B \cdot 0,866L$$

$$\Rightarrow H_B = 0,866P$$

Cremona



Barra		dir.	F.(P)	A(Ω)	de( $\frac{PL}{AE}$ )
CF	EC	—	-0,58	2	-0,29
FG	DE	—	0,58	2	0,29
CI	CH	\	-0,58	1	-0,58
IF	EH	/	0,58	1	0,58
FB	AE	\	-1,73	1	-1,73
BG	DA	/	-1,15	1	-1,15
IB	AH	—	-0,58	2	-0,29

b) Hallo los desplazamientos con un williot simple a partir de los puntos B y F

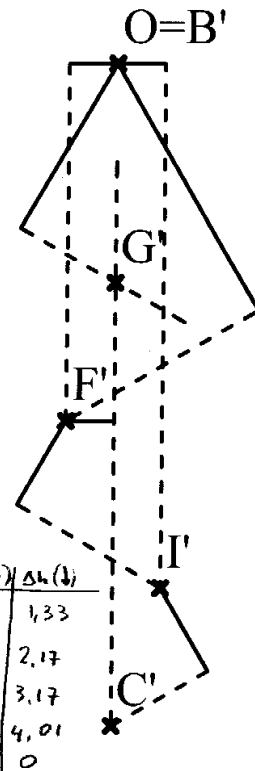
B es Fijo.

Para hallar F:

Como la estructura es simétrica, el punto C permanecerá sin desplazamientos horizontales

⇒ Como CF es horizontal, tengo el desplazamiento horizontal de F.

La posición final la obtengo por el vínculo de BF



	Δh(→)	Δh(↓)
G'	0	1,33
F'	-0,29	2,17
I'	0,29	3,17
C'	0	4,01
B'	0	0

( $\frac{PL}{AE}$ )